

Lamprey River Protected Instream Flow Study and Water Management Plan

Technical Review Committee
Meeting 13 February 2006

Normandeau Associates, Inc.
University of New Hampshire
University of Massachusetts

Assessment of Well Withdrawal Impacts on Surface Water

Do groundwater withdrawals reduce streamflow?

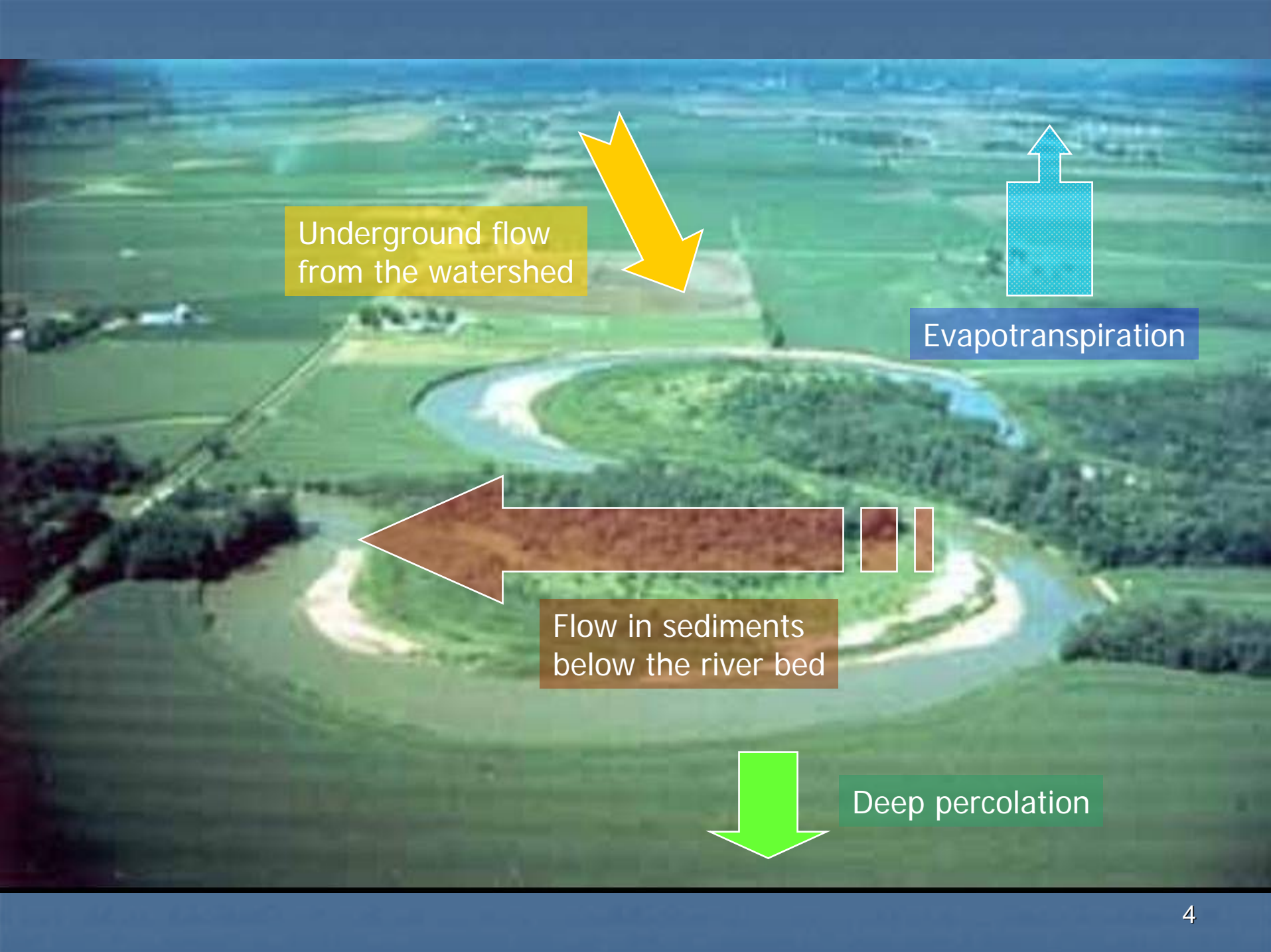
If so, can reductions to groundwater pumping be used at low flow times as one type of management strategy to meet instream flow needs?

How effective are such strategies in comparison to other withdrawal reduction strategies?

Do groundwater withdrawals reduce streamflow?

If the groundwater had not been pumped, where was it going?

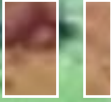

- Discharge to river, tributaries, or other surface waters
- Continued groundwater movement
- Deeper groundwater circulation

An aerial photograph of a river meandering through a green landscape. The river forms a large loop. Several hydrological processes are annotated with colored arrows and text boxes: a yellow arrow points from the watershed towards the river; a blue arrow points upwards from the river area; a brown arrow points from the river bed towards the left; and a green arrow points downwards from the river area. A legend with two brown boxes is located near the brown arrow.


Underground flow
from the watershed



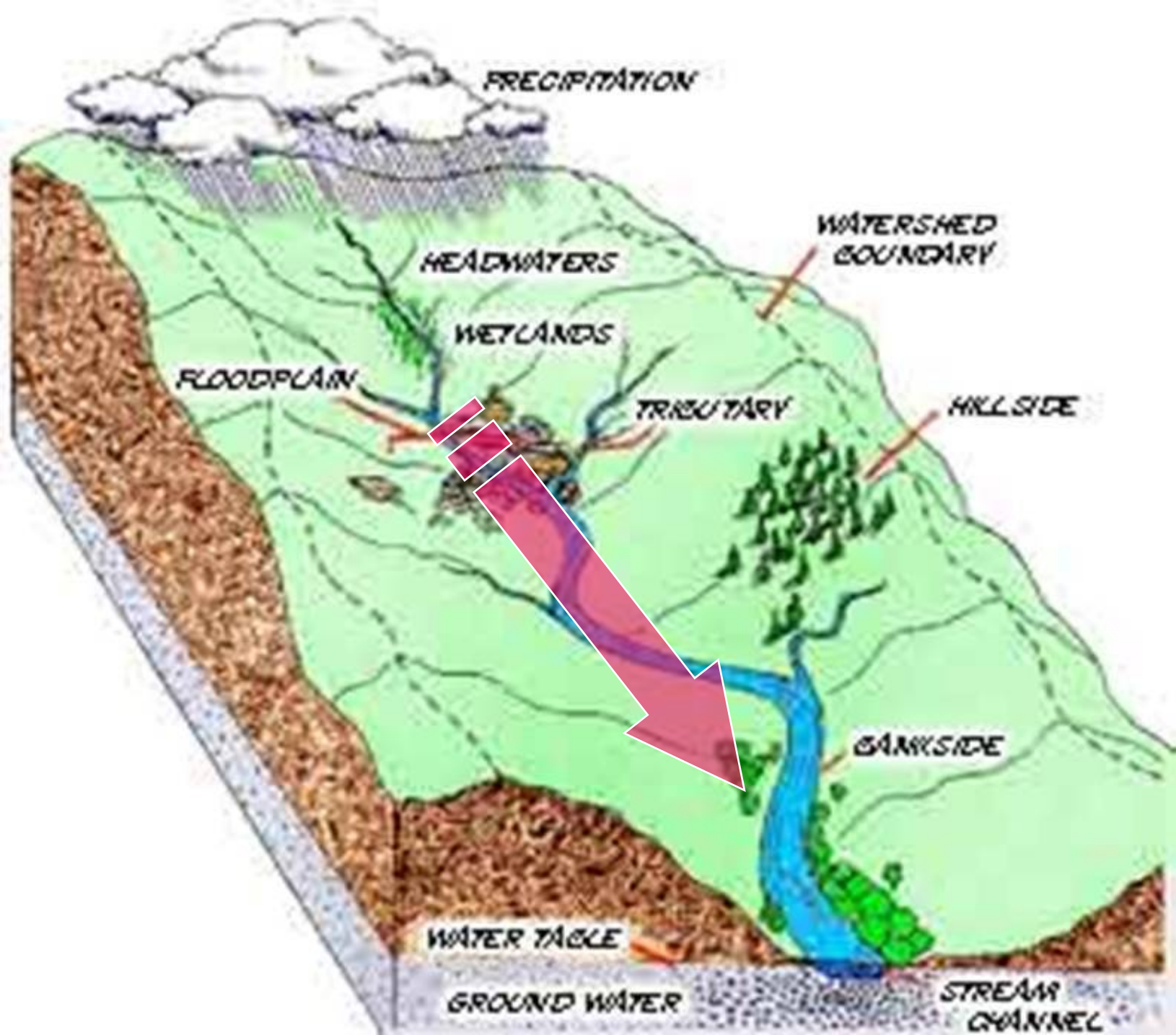
Evapotranspiration

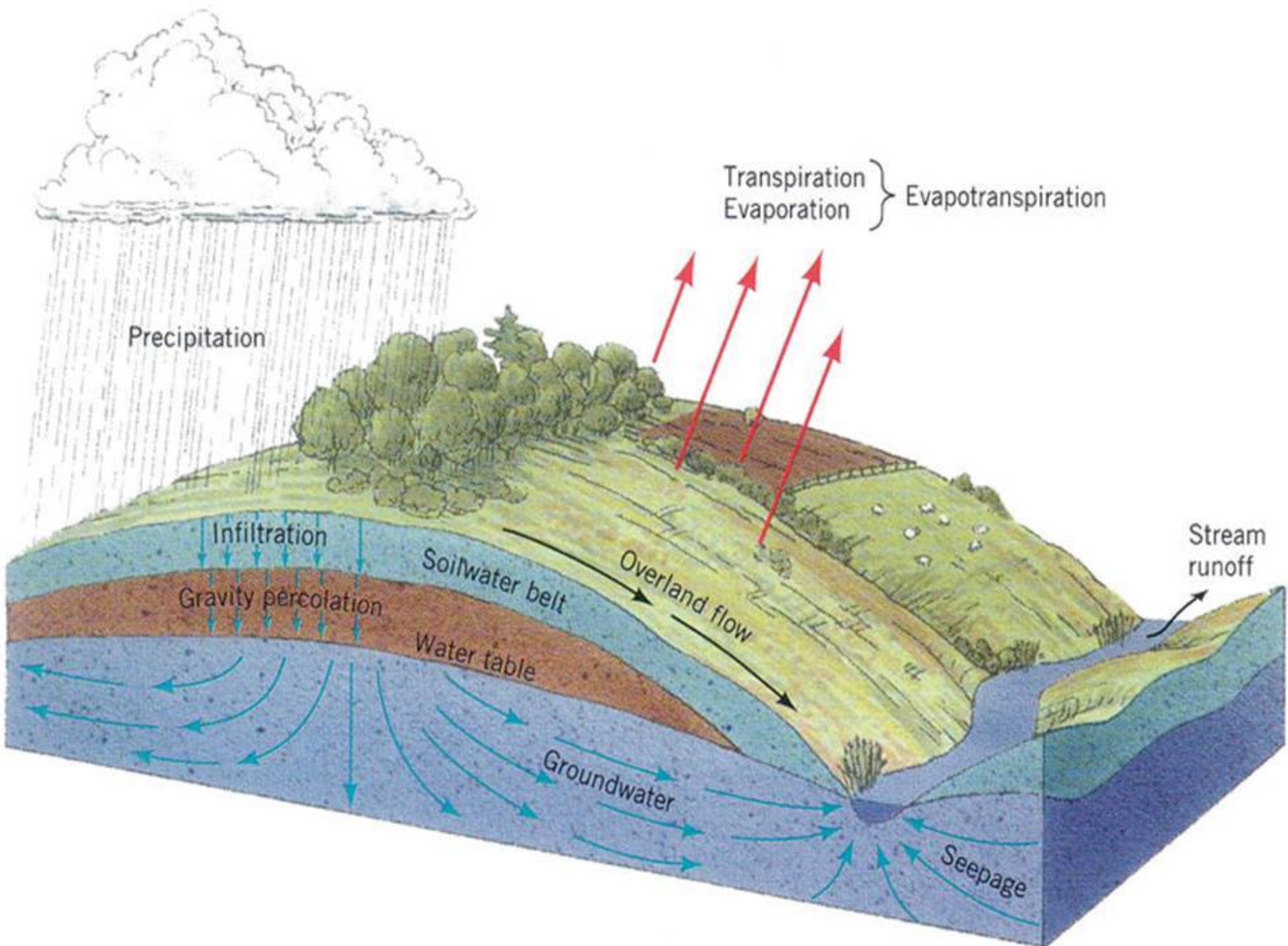


Flow in sediments
below the river bed



Deep percolation





Precipitation

Transpiration
Evaporation } Evapotranspiration

Infiltration

Gravity percolation

Soilwater belt

Water table

Groundwater

Overland flow

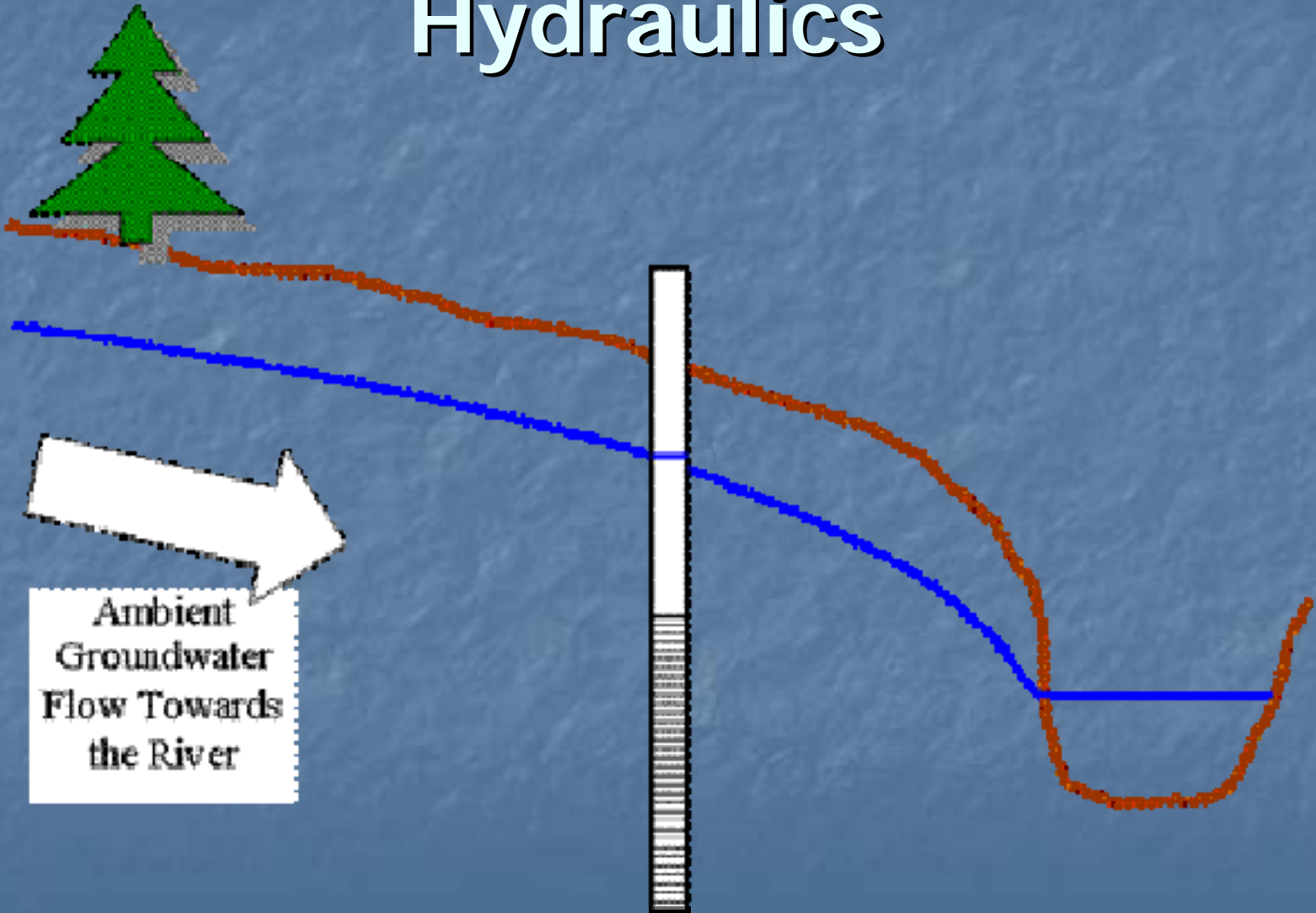
Seepage

Stream runoff

Can reductions to groundwater pumping be used at low flow times as one type of management strategy to meet instream flow needs?

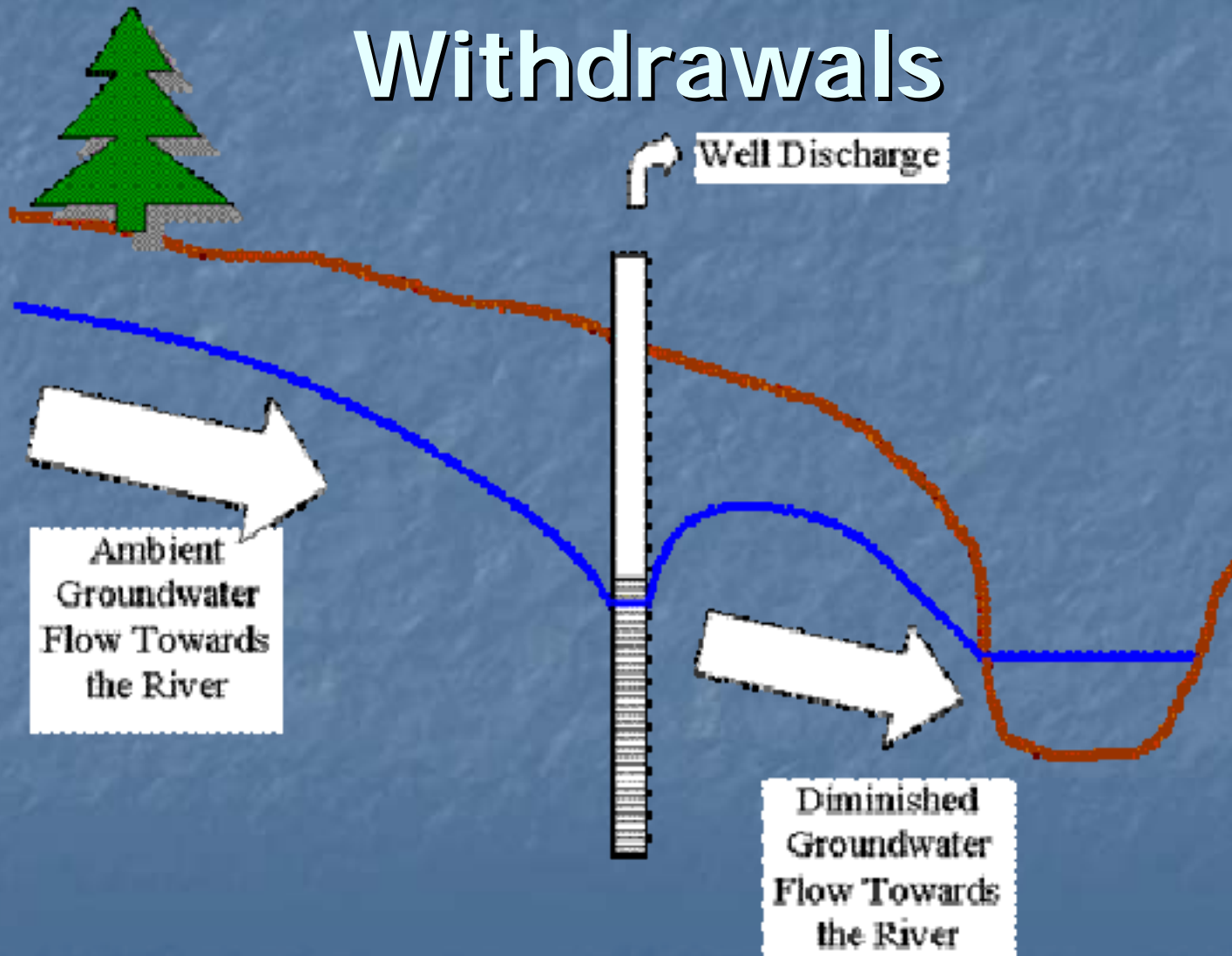
Are individual wells inducing river water recharge?

Ambient Groundwater Hydraulics

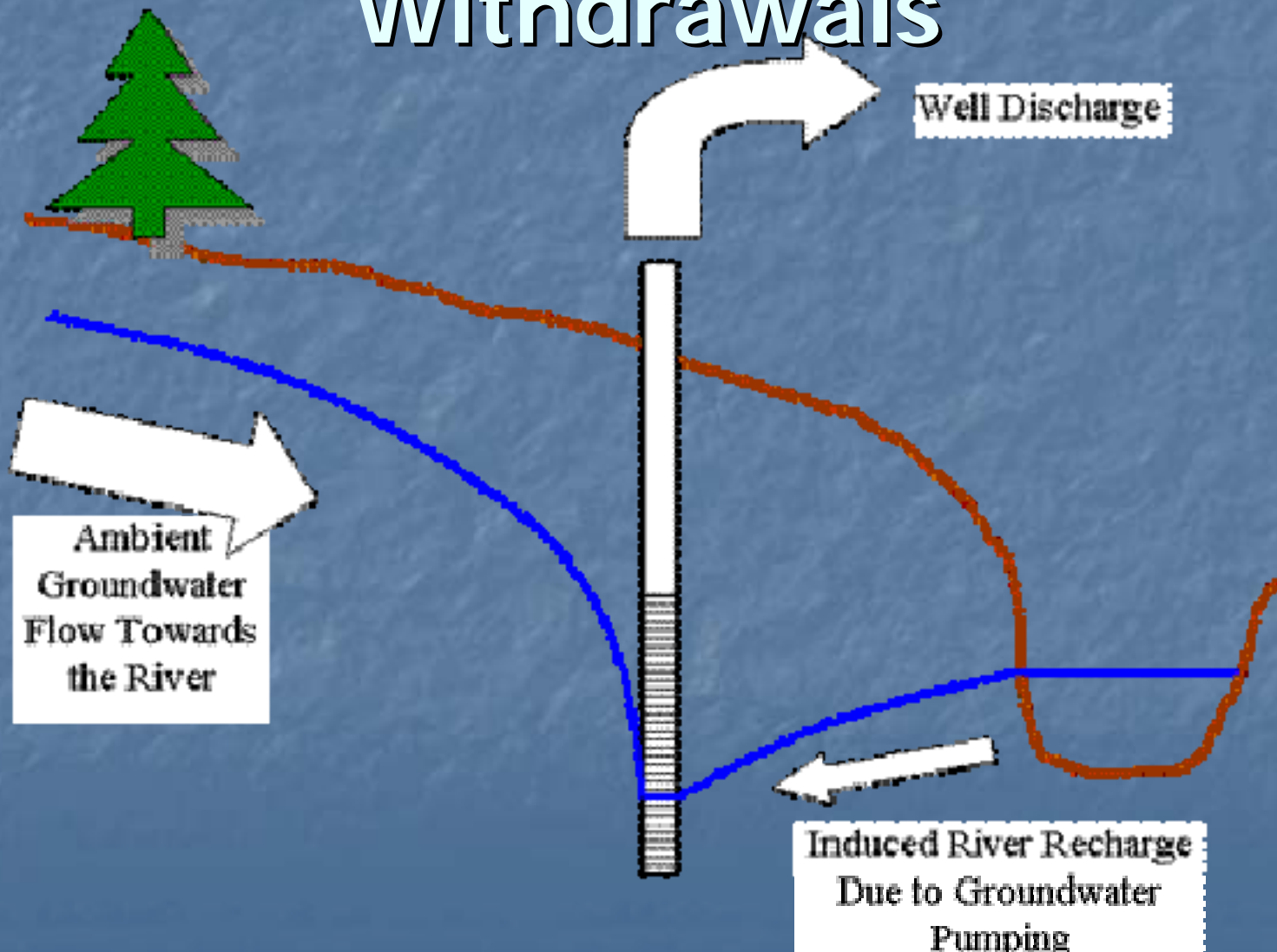


Ambient
Groundwater
Flow Towards
the River

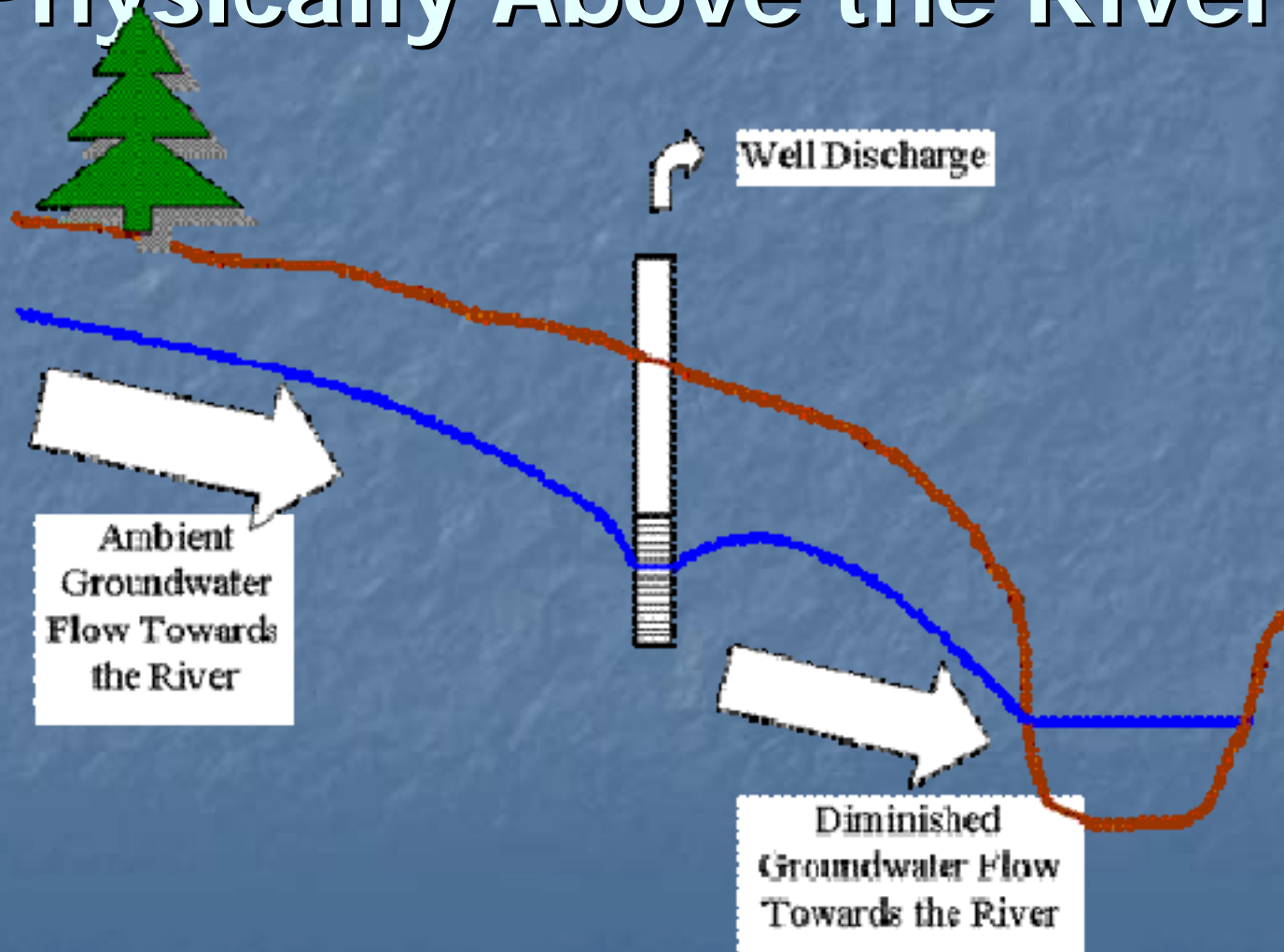
Effect of Relatively Small Quantities of Groundwater Withdrawals



Effect of Increasing the Quantity of Groundwater Withdrawals



Effect of Groundwater Pumping Even When Well is Physically Above the River



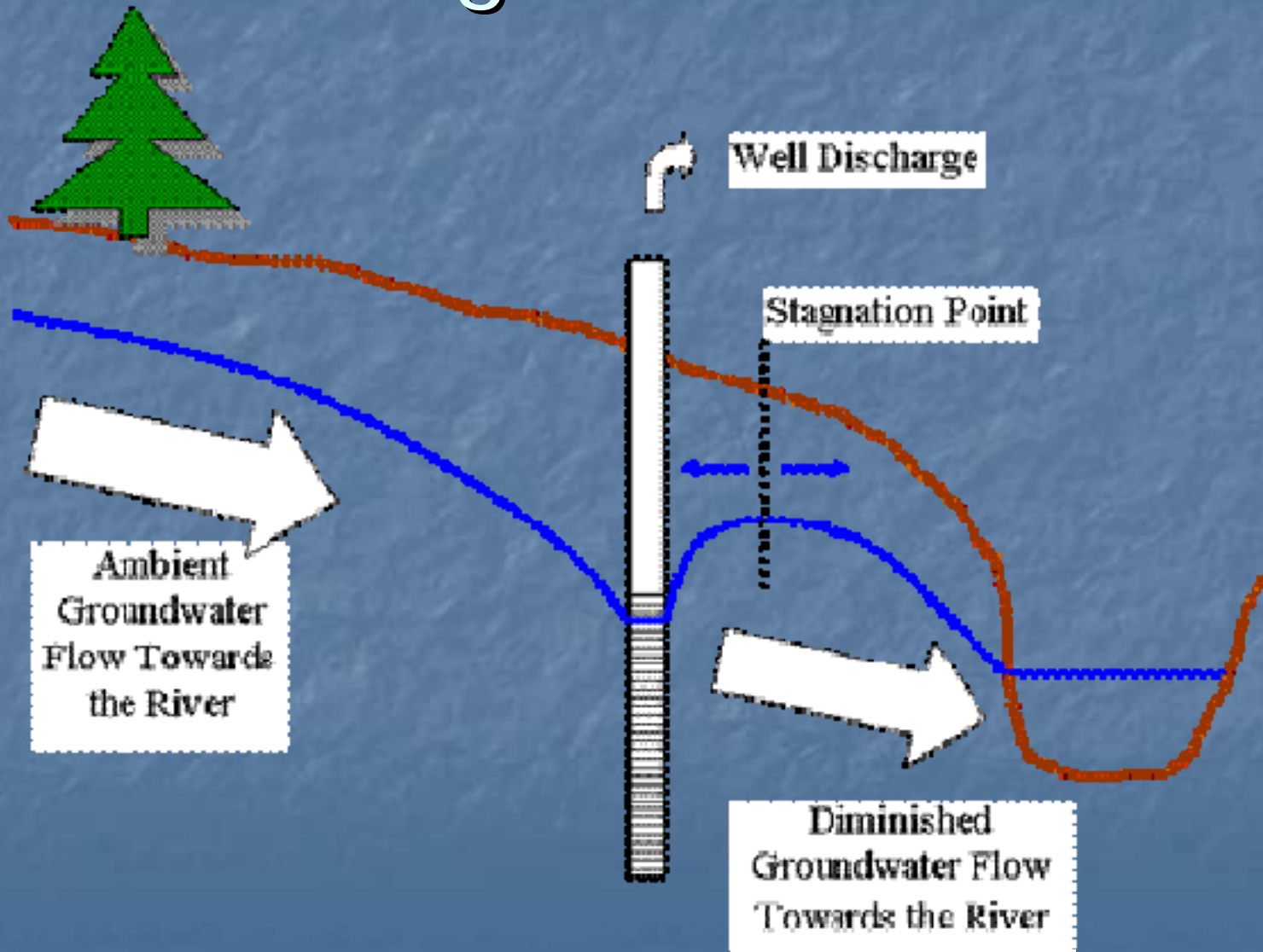
Induced Recharge

Since water resources operate on the watershed scale, in general, if the slope of the groundwater table is towards the river, then groundwater pumping has the likelihood of removing water that ultimately would have flowed in the river.

Induced recharge is surface water directly captured by groundwater pumping.

If the river discharge falls below the recommended ISF values, the most immediate flow control management strategy for groundwater is to address induced recharge.

Stagnation Point



Short Term Groundwater Management

When the stagnation point reaches the river, the well induces recharge from the river. Therefore a short term management strategy to improve (increase) river flow is to halt induced recharge. This would be accomplished by reducing the well pumping rate.

Methods for Evaluating Induced Recharge

1. Existing Study

2. Analytical Estimation

3. Field Measurements - Office

4. Field Measurements

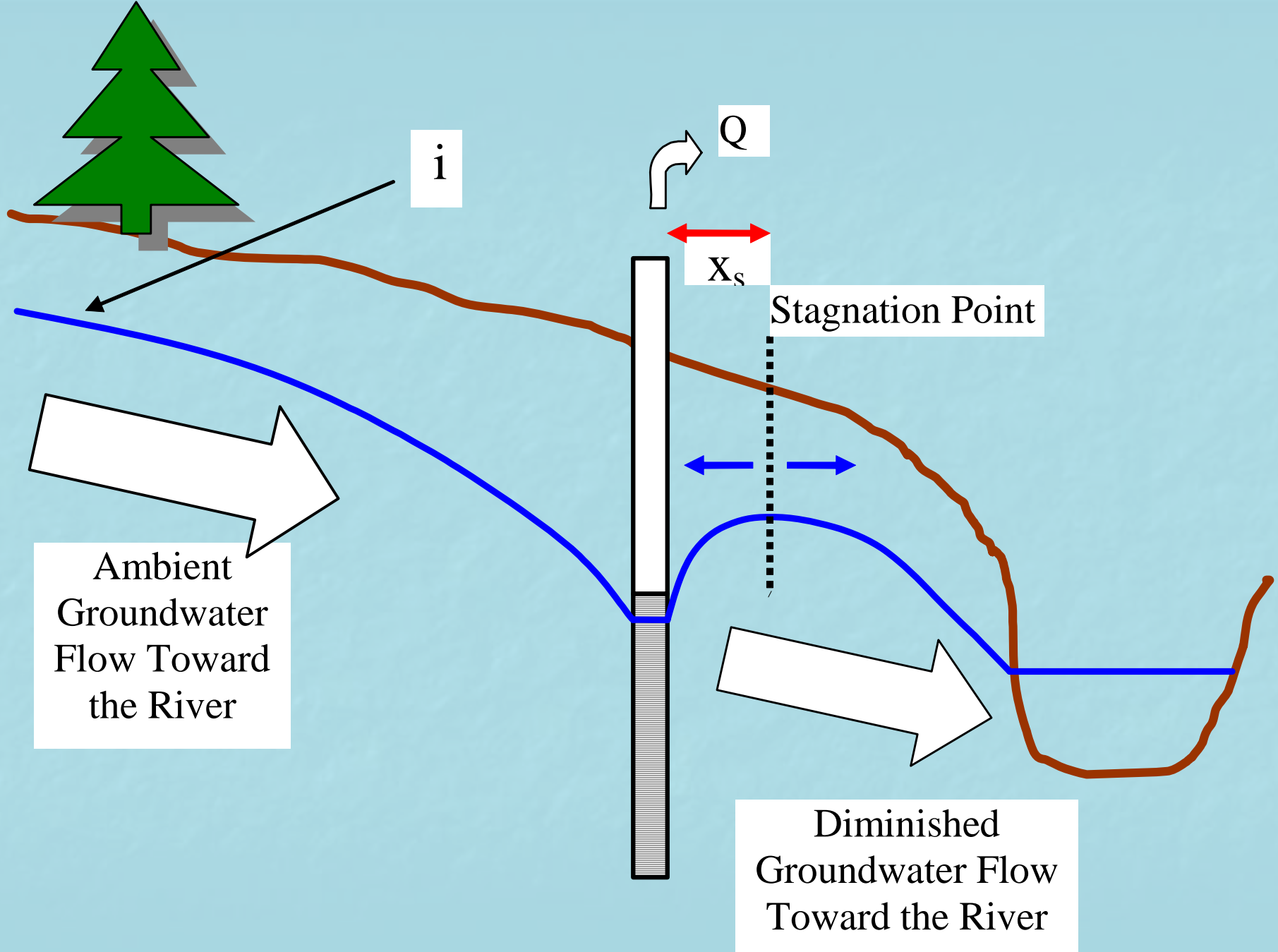
- **Miniature Piezometers or Wells**
- **Seepage Meters**
- **Tracers**
- **Pumping Tests**

Existing Study

Reports submitted for large
groundwater permit

Analytical Estimates

Use map or other existing aquifer information along with pumping information to estimate x_s and Q_s



Methodology for Well Pumping

Distance from well to stagnation point = x_s (feet)

$$x_s = \frac{Q}{2\pi T i}$$

Q = well pumping rate (cubic feet per day)

T = aquifer transmissivity (feet per day)

i = the slope to the groundwater table
(dimensionless)

Groundwater ISF Strategy

Compute well pumping rate to achieve x_s
(this is Q_s)

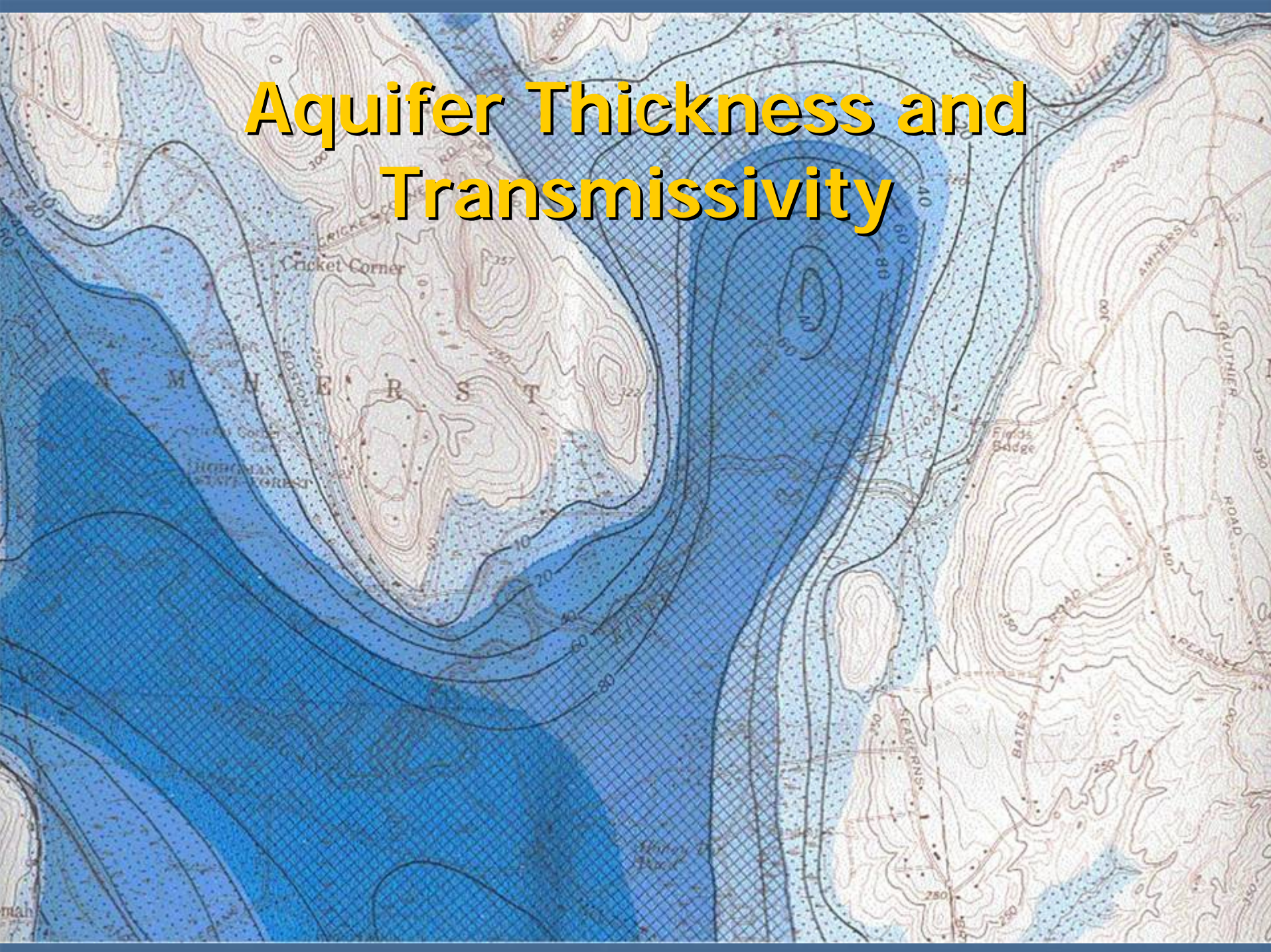
Compare Q_s to actual well pumping (Q_p)

If :

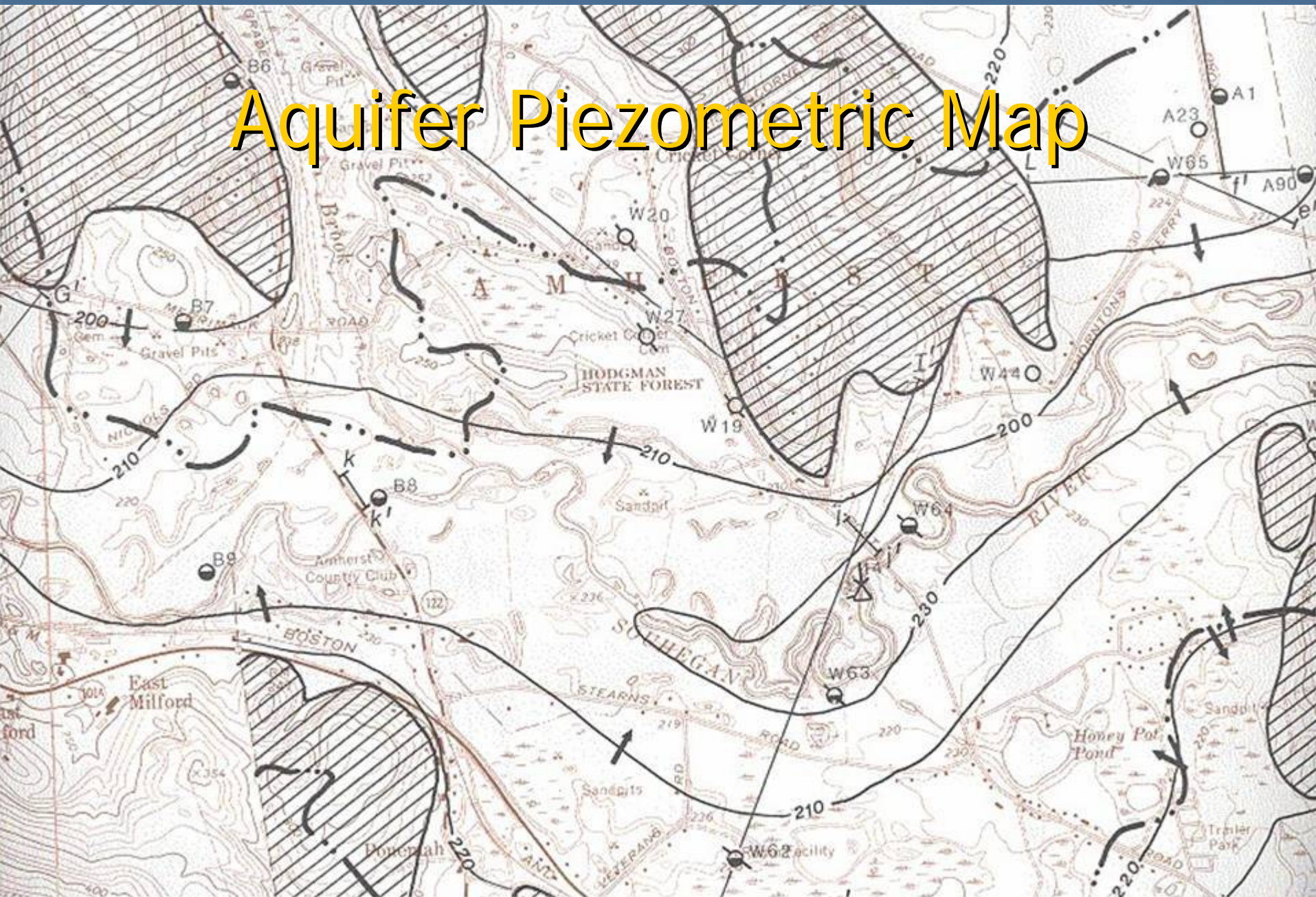
$Q_p \leq Q_s$ continue pumping

$Q_p > Q_s$ reduce pumping to Q_s

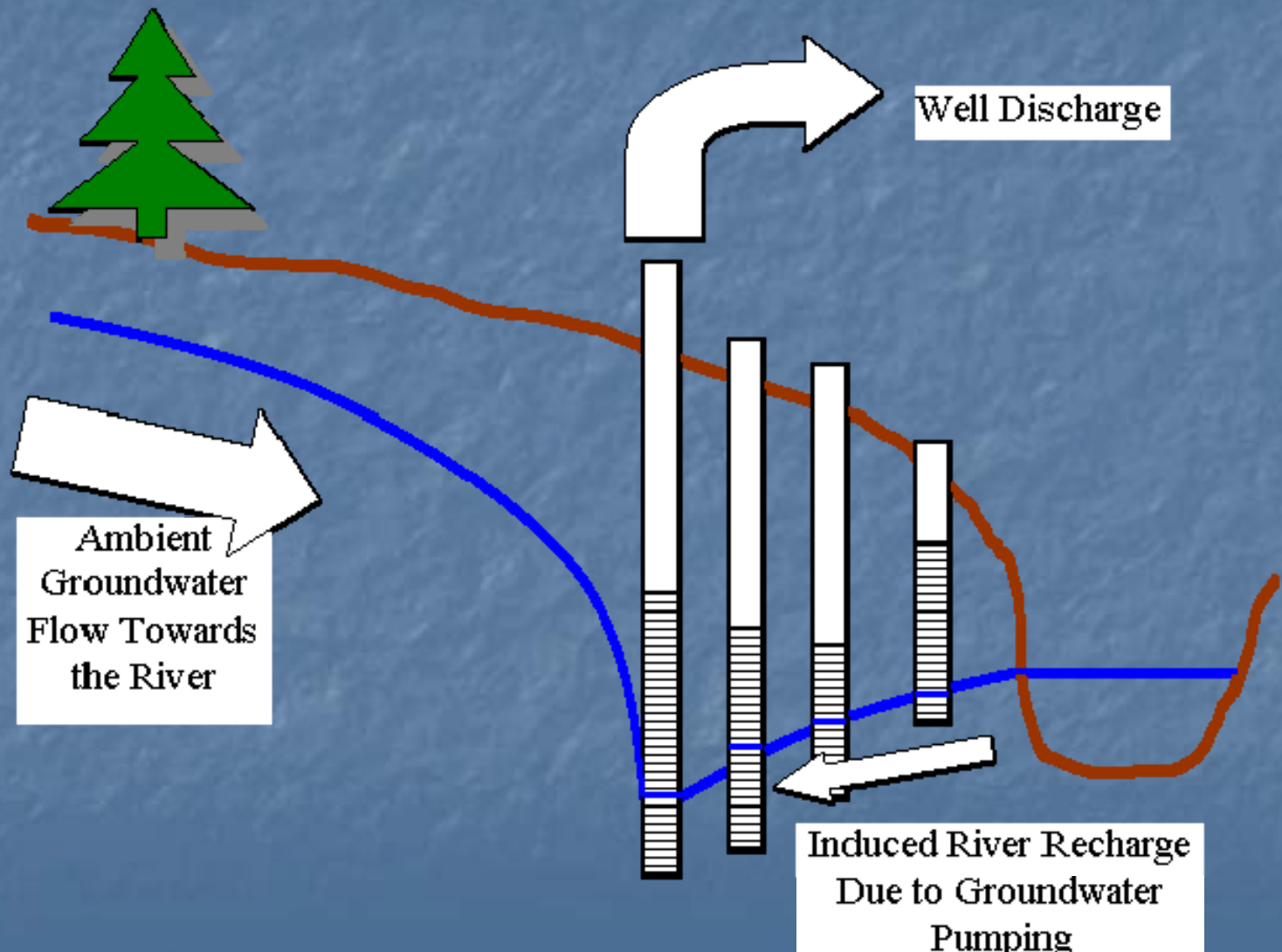
Aquifer Thickness and Transmissivity



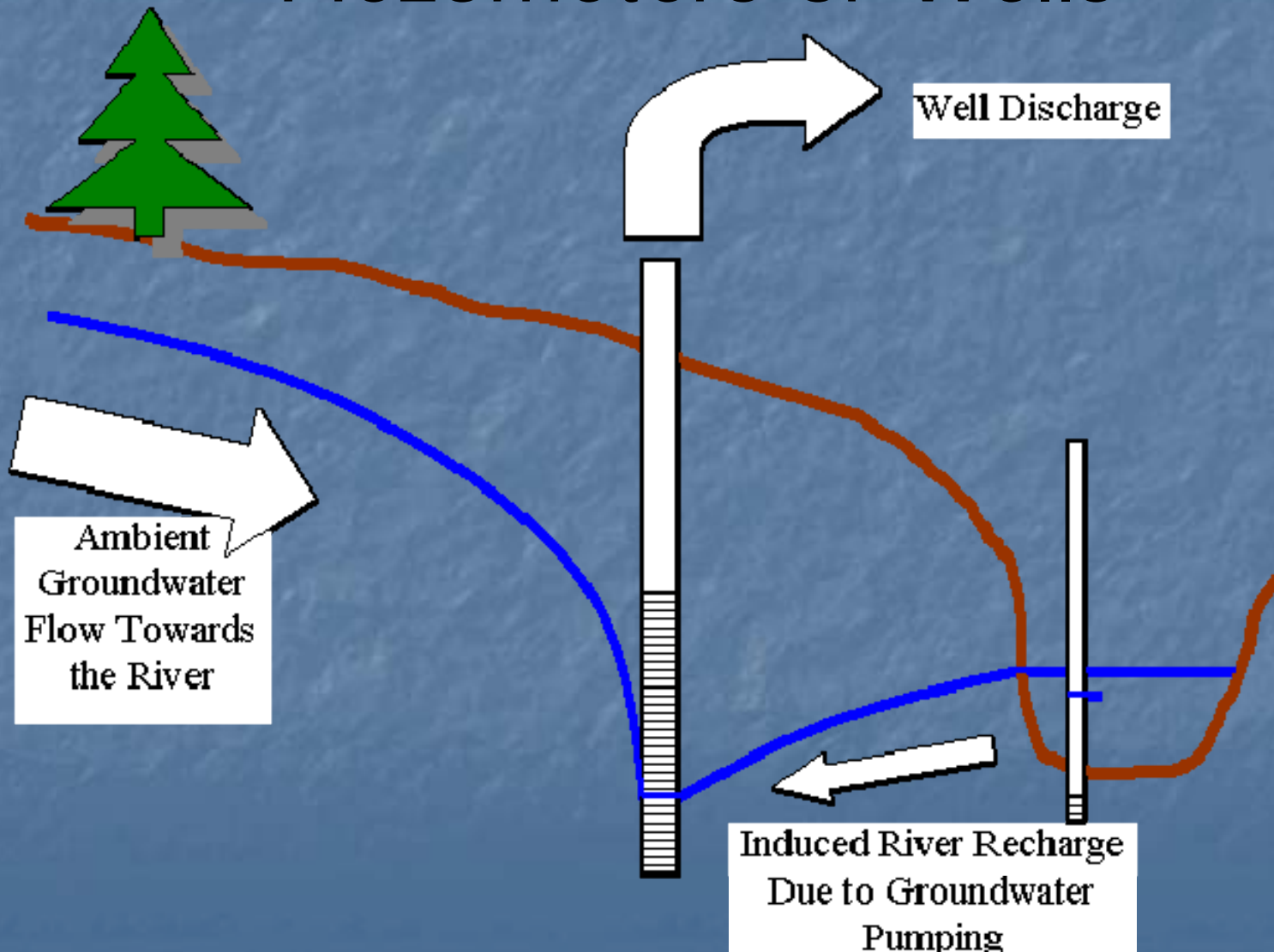
Aquifer Piezometric Map



Field Measurements - Office



Field Measurements – Miniature Piezometers or Wells





Case Study

Water supply well (Town of Pembroke, NH)
along the Soucook River

Site Data

- Well construction – summer 1993
- Pumping Test – November, 1993
 - 650 gpm
 - $T = 3800 \text{ ft}^2/\text{day}$
 - $S = 0.1$
 - $i = 0.03$
- Permitted Yield – 505 gpm

Explanation

- Municipal Water Supply Well
- Private Well
- ⊗ Registered known or potential contamination source

— aquifer boundary

--- watershed boundary

Scale 1:24000

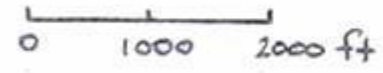
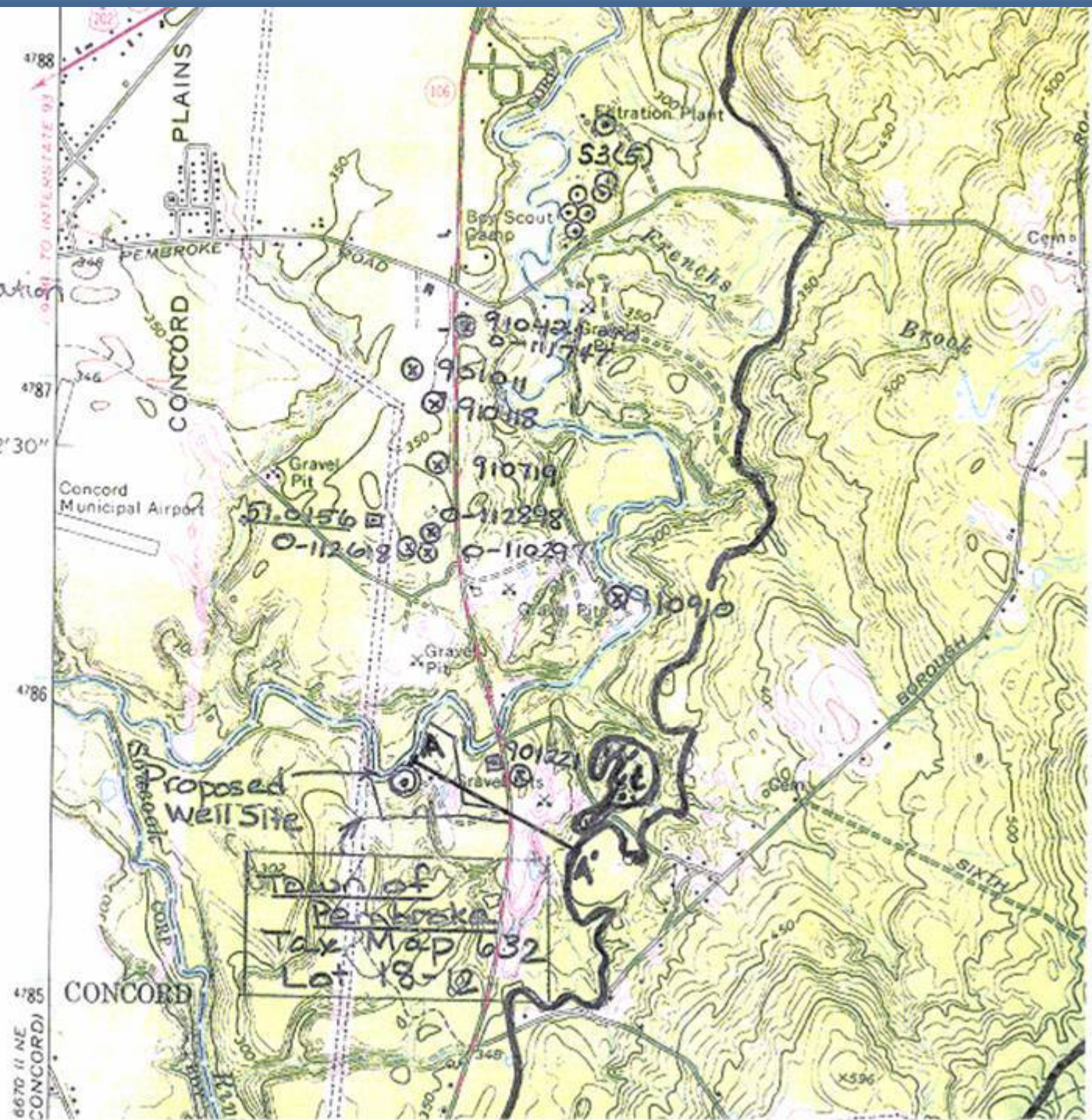


Figure 1:
Site Location and
Hydrologic Boundaries



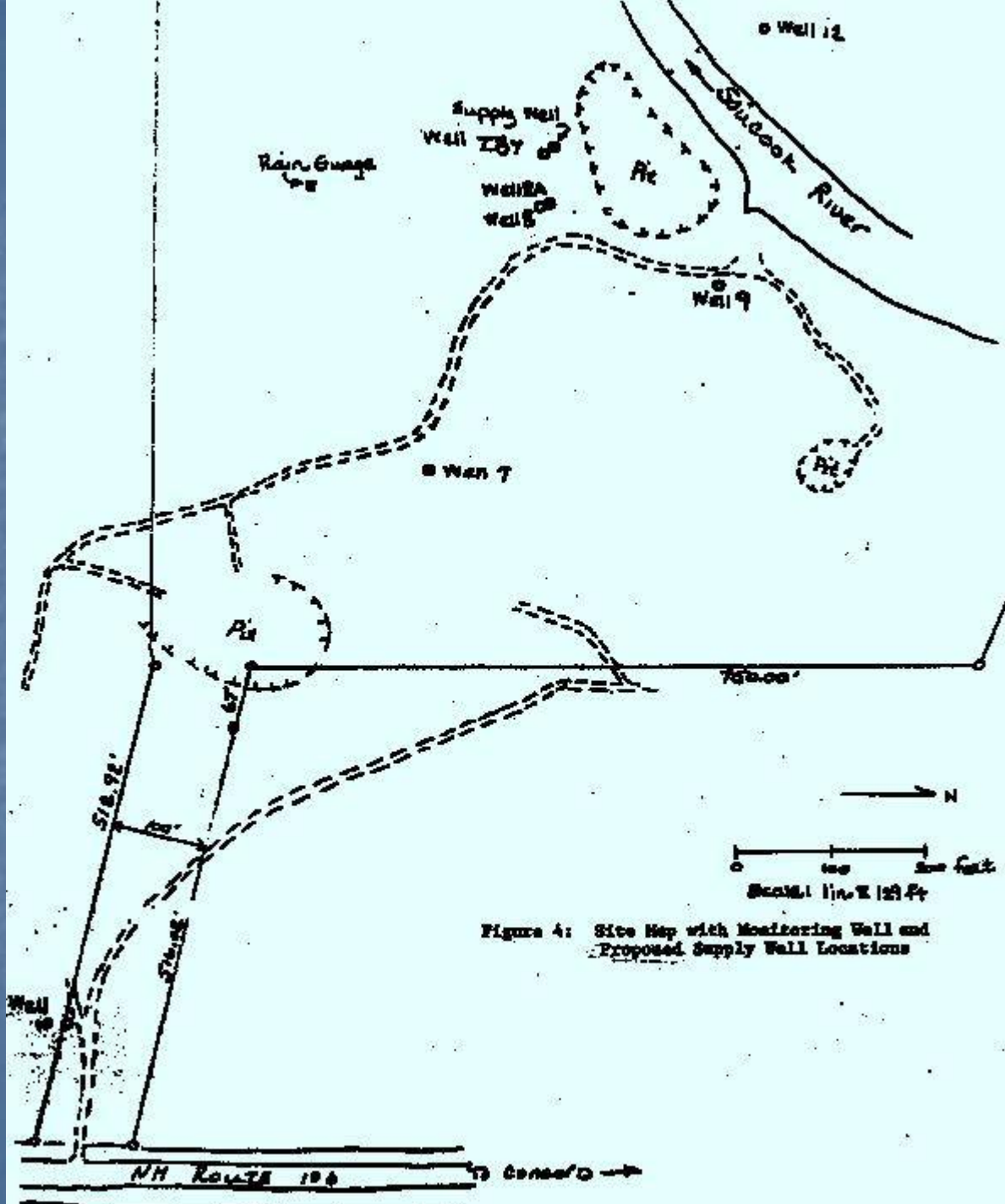
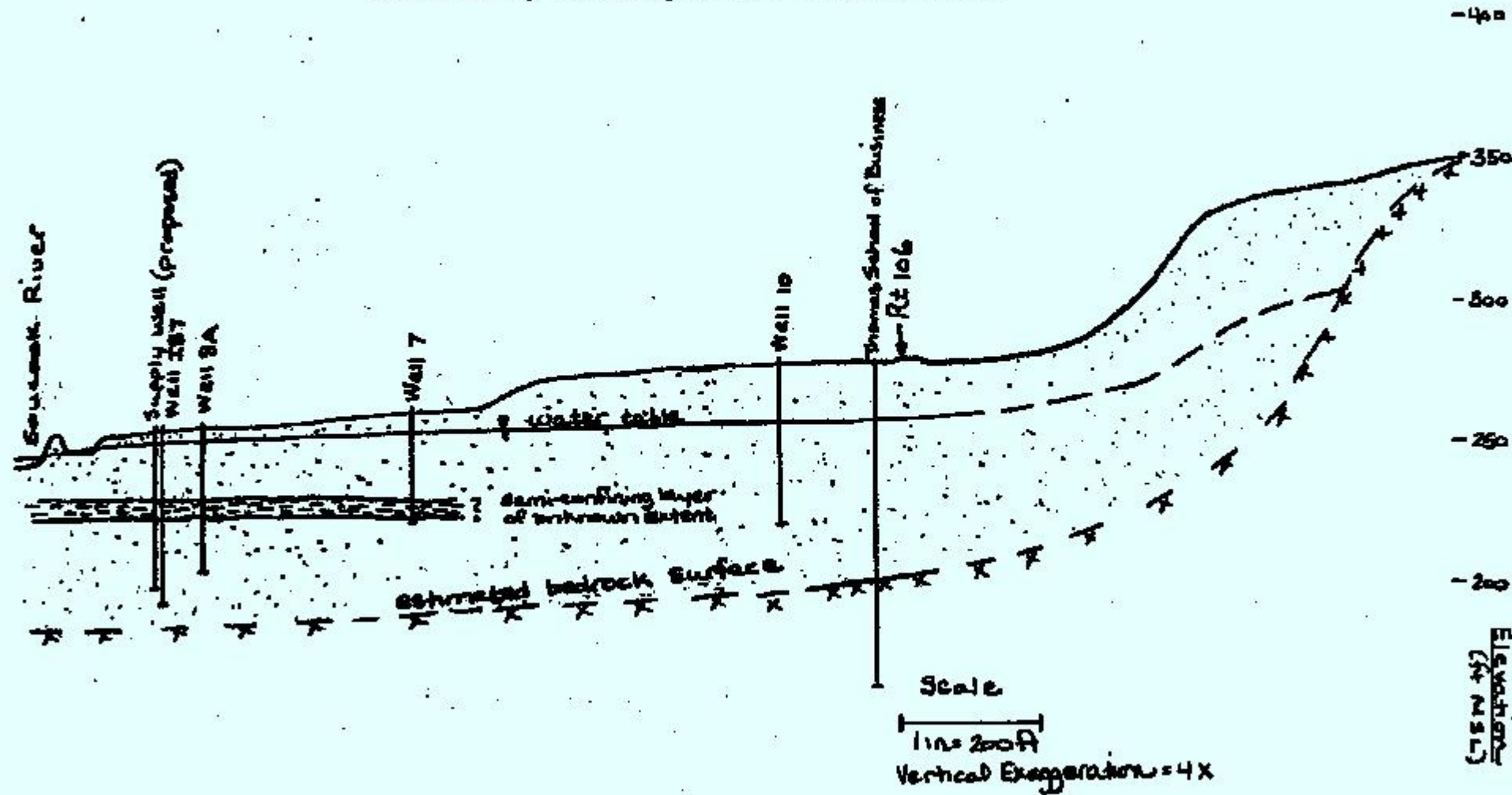


Figure 4: Site Map with Monitoring Well and Proposed Supply Wall Locations

Figure 3: Estimated Geologic Cross-Section of the Stratified Drift Aquifer in the Vicinity of the Proposed Well Site, Pembroke, NH



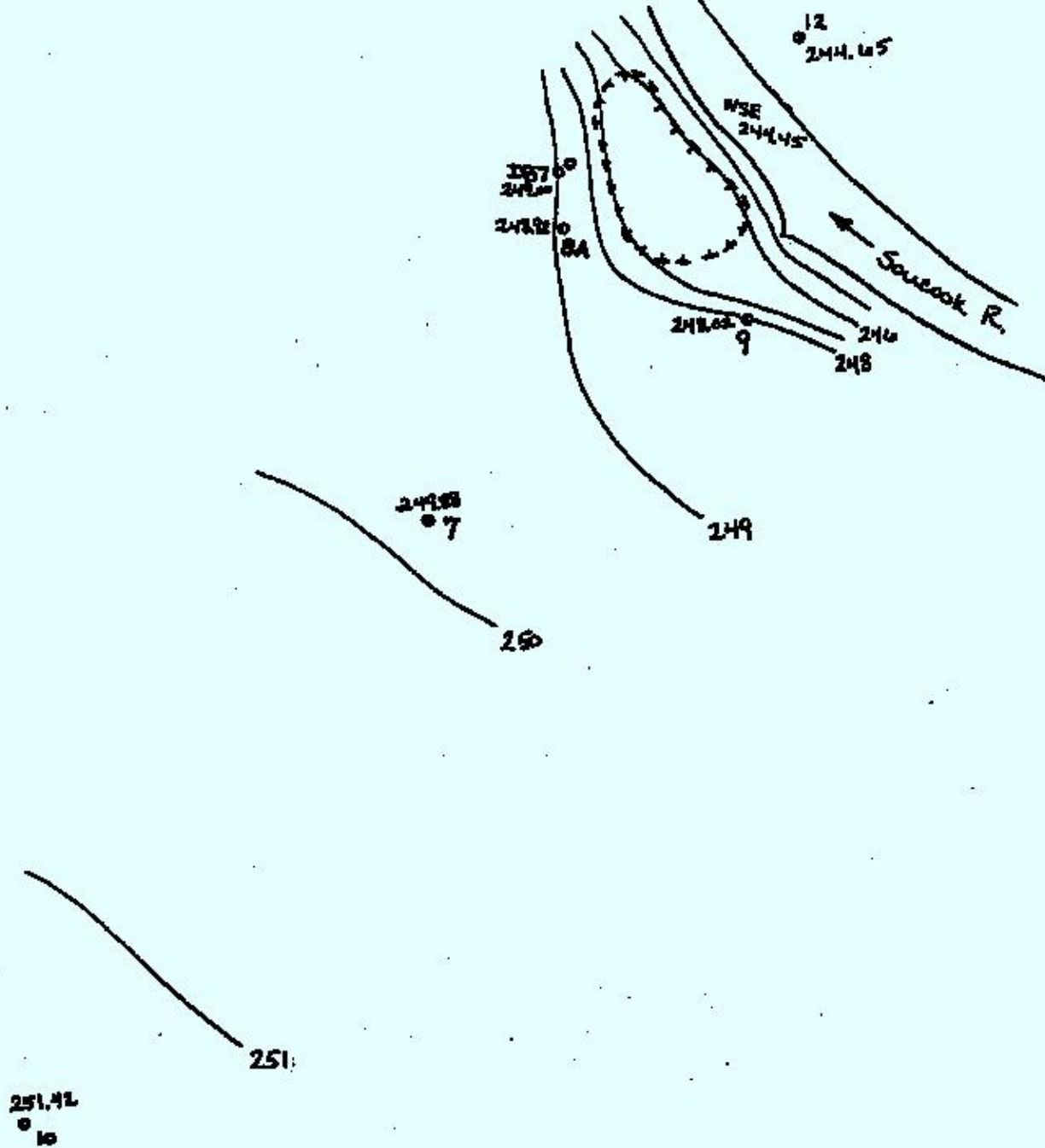


Figure 9a: Piezometric Contours prior to

Analytical Method

- $Q = 650$ gpm
 - $T = 3800$ ft²/day
 - $S = 0.1$
 - $i = 0.03$
 - Distance from well to river = 140 ft.
-
- $x_s = 175$ ft
 - $Q_s = 520$ gpm

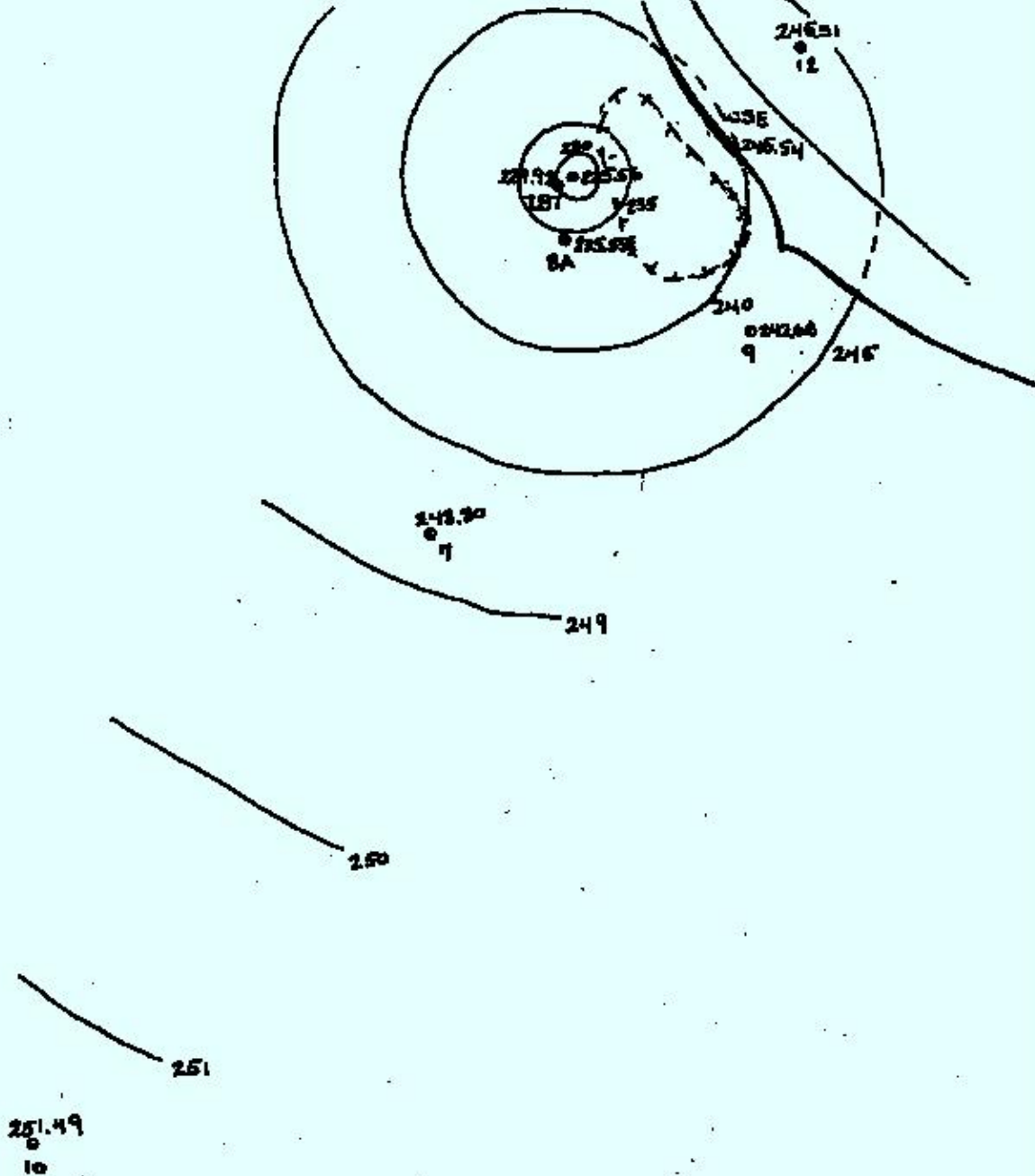


Figure 9b: Piezometric Contours after 100.5 hours of pumping (11/20/93 12:30 pm)

If ISF can be aided by reductions
in groundwater pumping,

“How long does it take for
the induced recharge to cease?”

Ambient groundwater velocities

SLOW

Velocities in the cone of depression
near to the pumping well

FAST

Ambient Groundwater Velocity

$$V = \frac{T * i}{B * \theta}$$

$$t = \frac{L}{V}$$

Example

Transmissivity = 8,000 ft²/d

Groundwater gradient = 0.01

Saturated thickness = 60 ft.

Effective porosity = 0.28

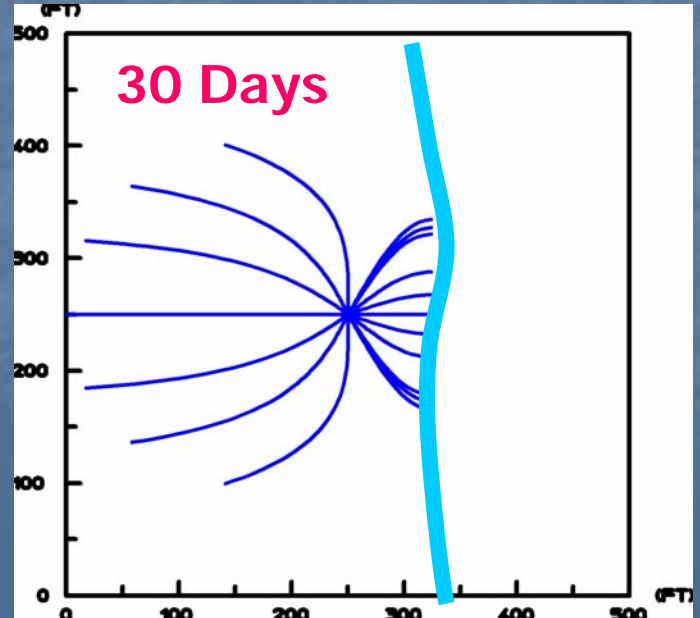
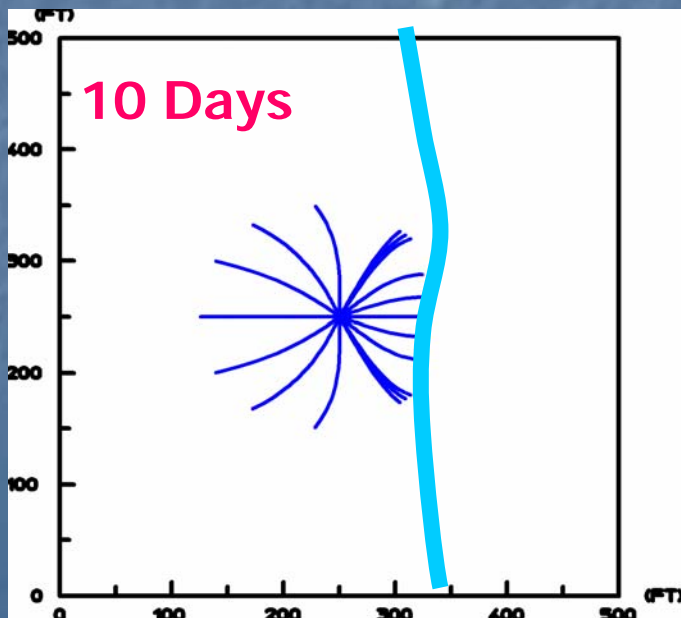
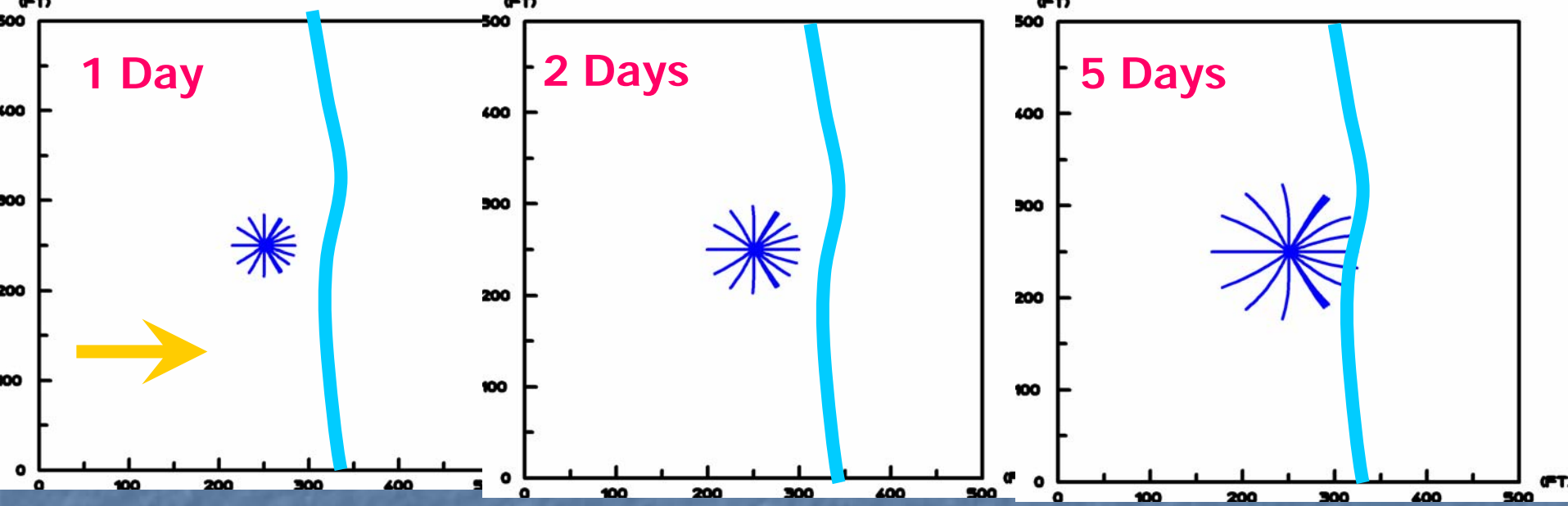
Distance to river = 100 ft.

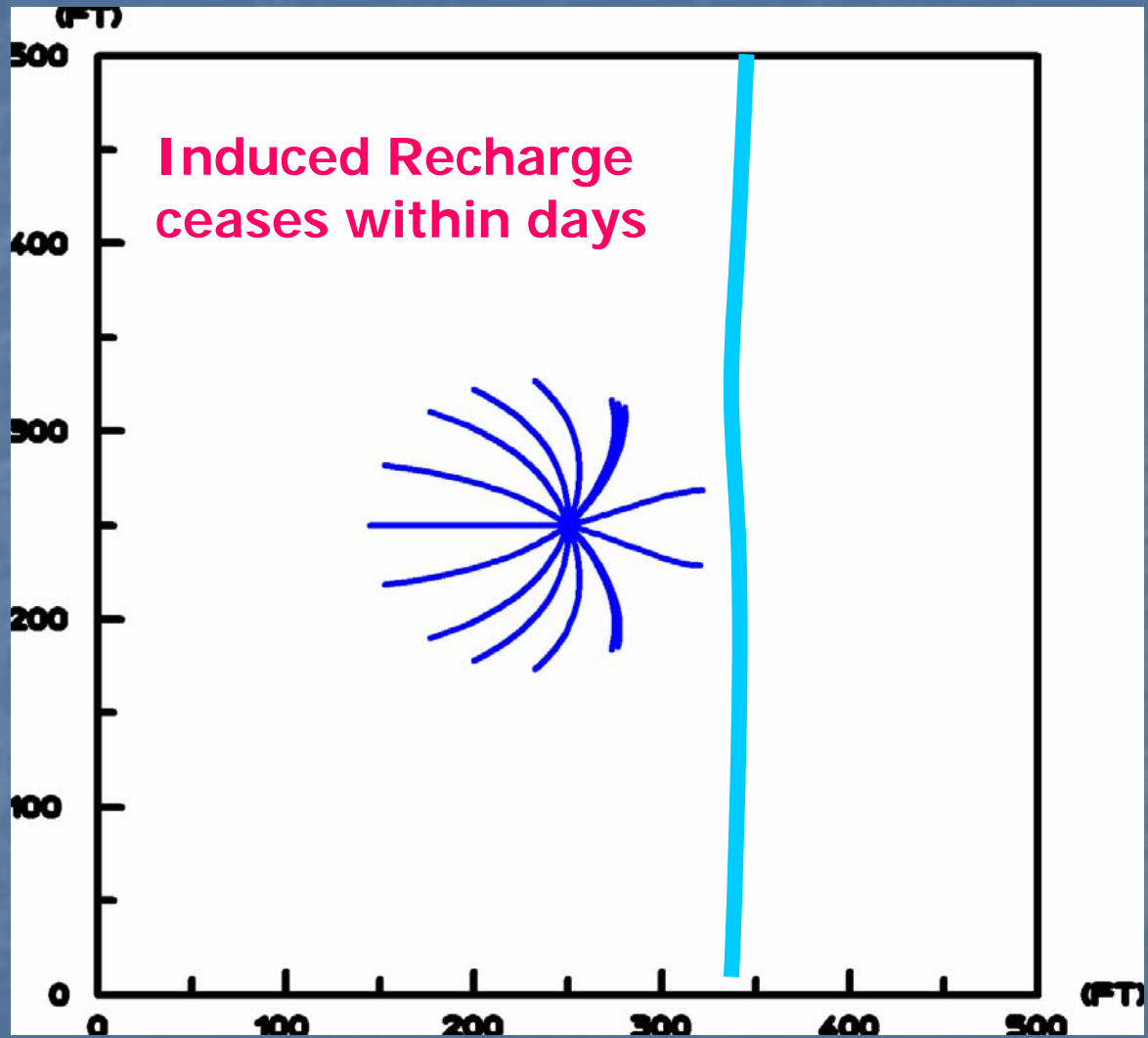
$$V = 4.8 \text{ ft/d}$$

$$t = 21 \text{ days}$$

Induced recharge groundwater velocities in the cone of depression and near to the pumping well can be extremely fast since the gradient increases dramatically and the saturated thickness is smaller

$$V = \frac{T * i}{B * \theta}$$





Souhegan River Results

| Well ID | Distance to River (ft) | Land Slope | Groundwater Slope | Transmissivity (sq. ft./day) |
|----------------|-----------------------------------|-------------------|--------------------------|---|
| A | - | 0.037 | 0.05 | 6,000 |
| B | 84 | 0.031 | 0.016 | 3,000 |
| C | 529 | 0.036 | 0.02 | 2,000 |
| D | 110 | 0.01 | 0.01 | 8,000 |
| E | 99 | 0.019 | 0.0133 | 2,000 |
| F | 75 | 0.055 | 0.009 | 1,000 |
| G | 994 | - | - | - |
| H | 306 | - | - | - |
| I | 273 | 0.006 | 0.0114 | 8,000 |
| J | 140 | 0.006 | 0.011 | 1,000 |
| K | 100 | - | - | - |
| L | 89 | 0.031 | 0.016 | 3,000 |
| M | 92 | 0.0005 | 0.032 | 6,000 |
| N | 849 | 0.013 | 0.02 | 6,000 |
| O | 97 | 0.015 | 0.01 | 8,000 |
| P | 213 | 0.015 | 0.02 | 8,000 |
| Q | 74 | 0.015 | 0.01 | 8,000 |
| R | 110 | 0.01 | 0.01 | 8,000 |
| S | 316 | - | - | - |
| T | 668 | 0.0129 | 0.0017 | 8,000 |

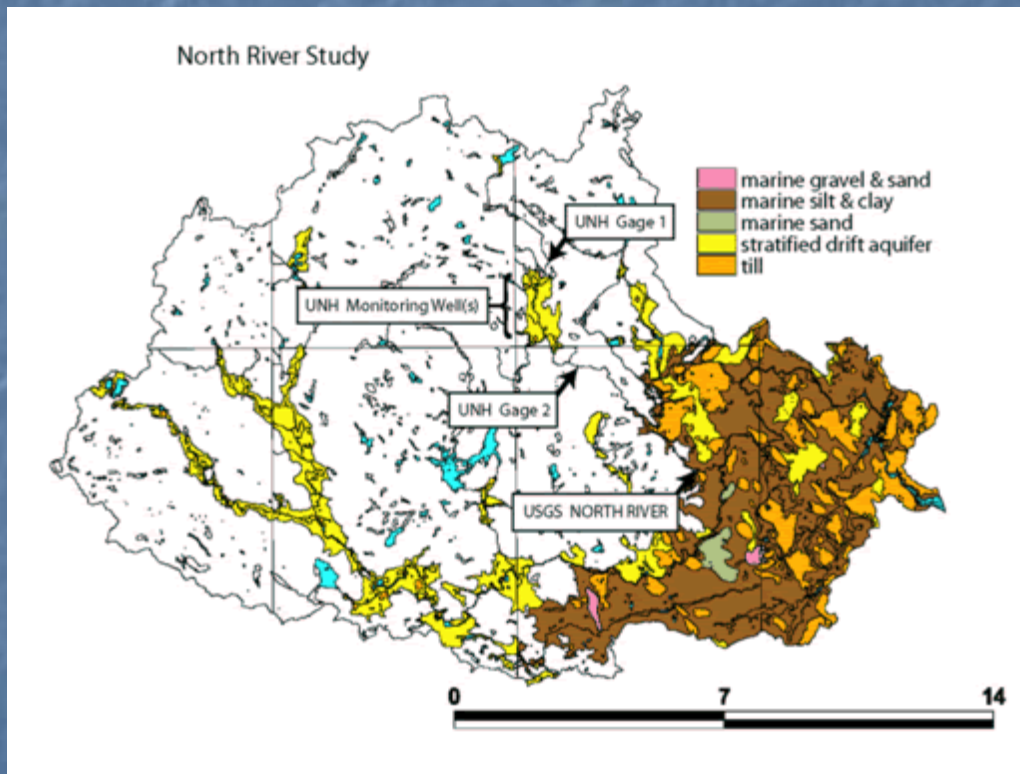
| Well ID | Maximum Reported Average Well Discharge (1,000 gal/day) | Max Flow Stagnation Distance (ft) | 2000-2004 Reported Average Well Discharge (1,000 gal/day) | Avg Flow Stagnation Distance (ft) | Actual Distance (ft) |
|----------------|--|--|--|--|-----------------------------|
| A | 0 | 0.0 | 0 | 0.0 | - |
| B | 31.0 | 13.7 | 20.0 | 8.9 | 84 |
| C | 62.0 | 33.0 | 39.8 | 21.2 | 529 |
| D | 0 | 0.0 | 0 | 0.0 | 110 |
| E | 111 | 88.8 | 41.5 | 33.2 | 99 |
| F | 20.8 | 49.2 | 10.6 | 25.1 | 75 |
| G | 0 | 0.0 | 0 | 0.0 | 994 |
| H | 0 | 0.0 | 0 | 0.0 | 306 |
| I | 2139 | 499.0 | 1701 | 396.9 | 273 |
| J | 0 | 0.0 | 0 | 0.0 | 140 |
| K | 0 | - | 0 | - | 100 |
| L | 31.0 | 13.7 | 20.0 | 8.9 | 89 |
| M | 156 | 17.3 | 37.5 | 4.2 | 92 |
| N | 312 | 55.3 | 96.0 | 17.0 | 849 |
| O | 215 | 57.2 | 96.5 | 25.7 | 97 |
| P | 700 | 93.1 | 470 | 62.5 | 213 |
| Q | 700 | 186.2 | 470 | 125.0 | 74 |
| R | 0 | 0.0 | 0 | 0.0 | 110 |
| S | 0 | 0.0 | 0 | 0.0 | 316 |
| T | 1656 | 2590.8 | 851 | 1331.4 | 668 |

Public Water Supply



- Durham withdraws water from above Wiswall Dam
- Newmarket has a surface water withdrawal
- Potential well sites for Lee and Newmarket along designated reach

Groundwater Resources



M. Davis, UNH

- Stratified drift aquifers (sand & gravel)
- Drinking water sources (Raymond, potentially Epping, Durham, and Newmarket)

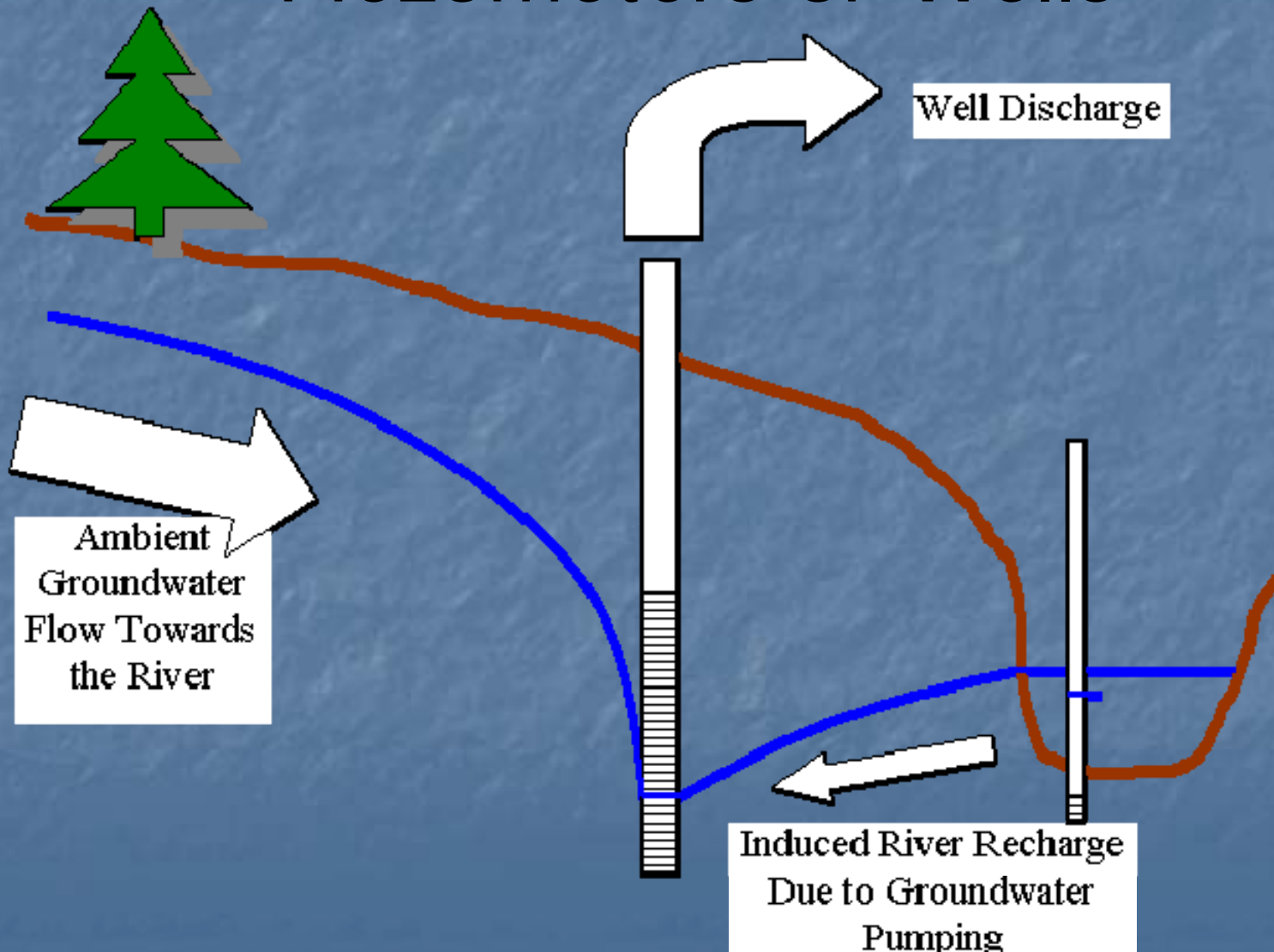
| Well ID | 2000-2004 Reported Average Well Discharge (1,000 gal/day) | Flow to Achieve Stagnation at River (1,000 gal/day) | Possible Average Flow Reduction (1,000 gal/day) |
|----------------|--|--|--|
| I | 1,701 | 1,170 | 531 |
| Q | 470 | 278 | 192 |
| T | 851 | 427 | 424 |

**The preceding were the results of
numerical estimates (office techniques)**

Field Techniques

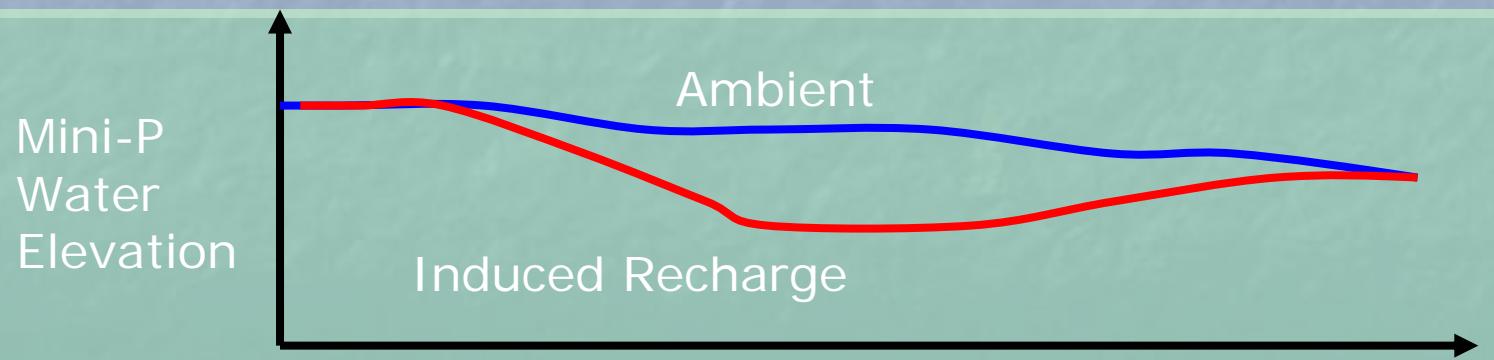
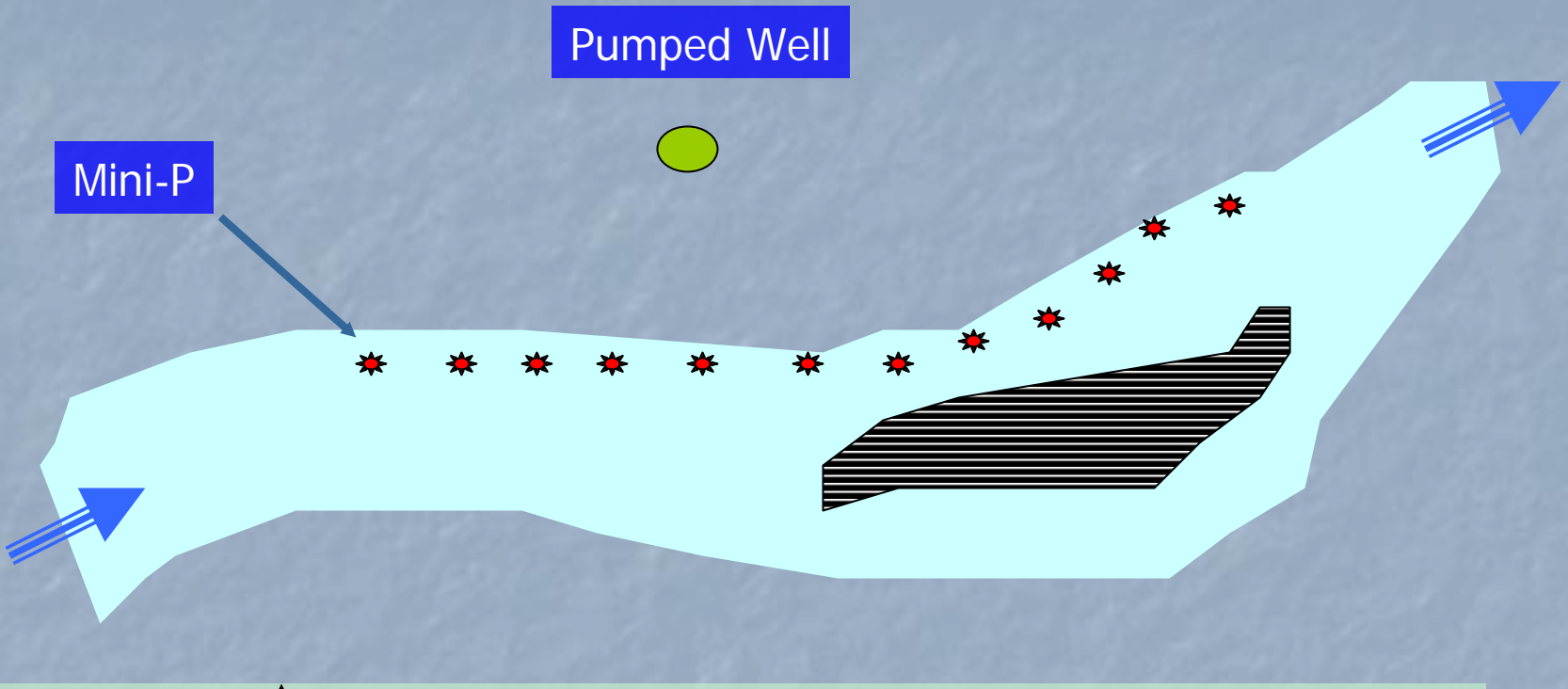
- Pumping Tests
- Monitoring
- **Miniature Piezometers**
- Seepage Flux Meters
- Tracer Tests

Field Measurements – Miniature Piezometers or Wells



Poorly-groomed
field tech





Field-Measured Vertical Gradients

(negative indicates downward flow)

| Well ID | Horizontal Groundwater Slope (USGS Maps) | Max. Vertical Groundwater Slope (Field) | Upstream Distance to Ambient Slope (ft) | Upstream Distance to Ambient Slope (ft) |
|----------------|---|--|--|--|
| B | 0.016 | 0.067 | 0 | 0 |
| I | 0.0114 | -1.74 | 400 | 800 |
| L | 0.016 | 0.067 | 0 | 0 |
| P | 0.02 | -1.00 | 300 | 900 |
| Q | 0.01 | -1.00 | 300 | 900 |
| T | 0.0017 | -1.74 | 400 | 800 |

Wells I, Q, and T were predicted to exhibit induced recharge. Well P is very close to well Q. The field data verified the numerical predictions.

